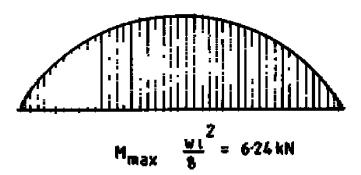
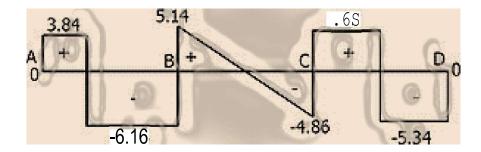
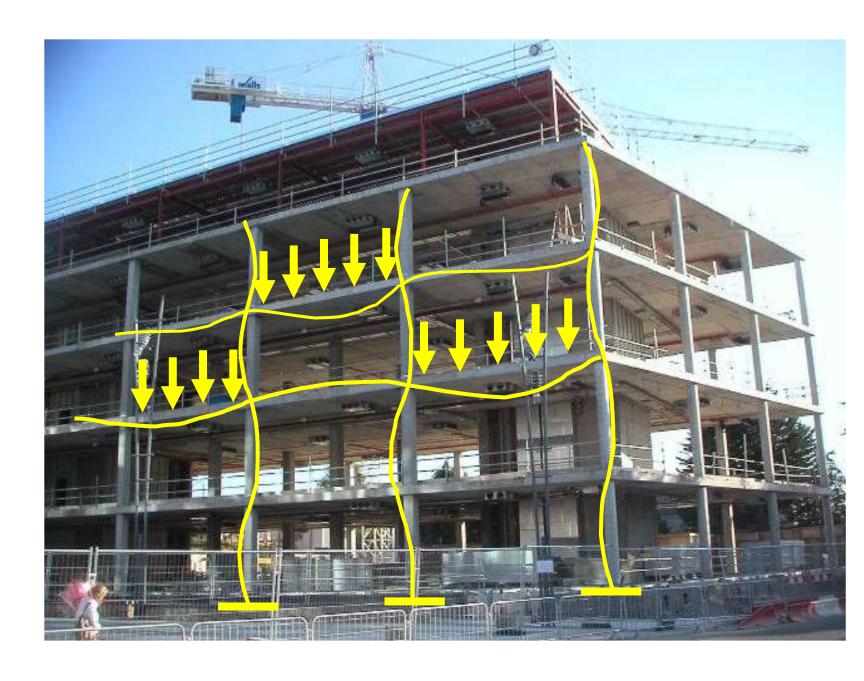
Shear Force & Bending Moment









INTRODUCTION

	Effects	Action
Loading	Shear Force	Design Shear reinforcement
Loading	Bending Moment	Design flexure reinforcement

SHEAR FORCE & BENDING MOMENT

- Introduction
 - Types of beams
 - Effects of loading on beams
 - The force that cause shearing is known as shear force
 - The force that results in bending is known as bending moment
 - Draw the shear force and bending moment diagrams

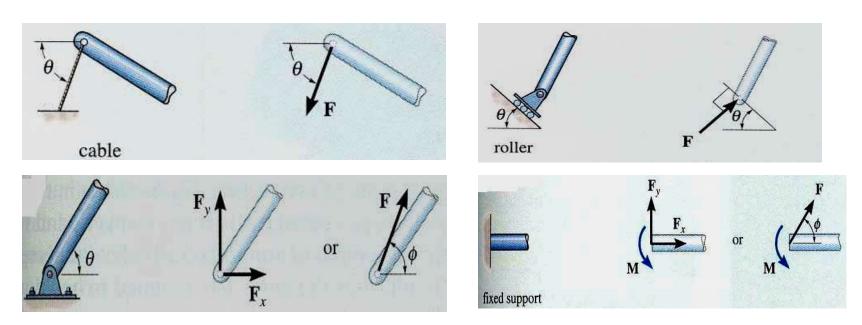
SHEAR FORCE & BENDING MOMENT

- Members with support loadings applied perpendicular to their longitudinal axis are called *beams*.
- Beams classified according to the way they are supported.





TYPES OF SUPPORT



As a general rule, if a *support prevents translation* of a body in a given direction, then *a force is developed* on the body in the opposite direction.

Similarly, if

rotation is prevented, a couple moment is exerted on the body.

SHEAR FORCE & BENDING MOMENT

Types of beam

a) Determinate Beam

The force and moment of reactions at supports can be determined by using the 3 equilibrium equations of statics i.e.

$$\Sigma F_x = 0$$
, $\Sigma F_y = 0$ and $\Sigma M = 0$

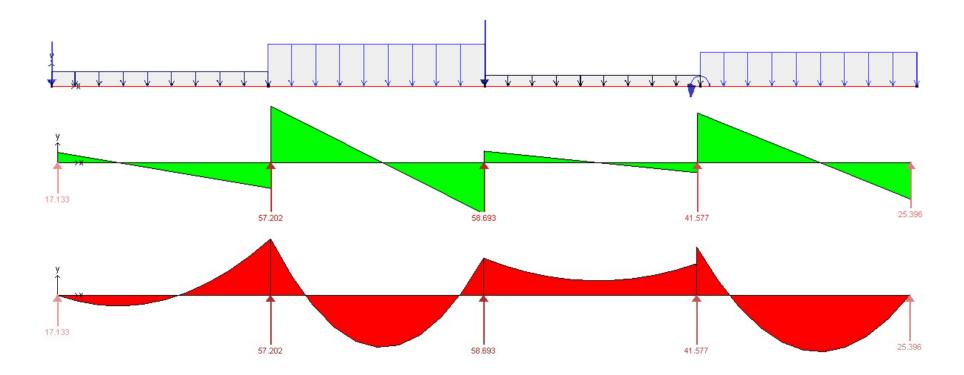
b) Indeterminate Beam

The force and moment of reactions at supports are more than the number of equilibrium equations of statics. (The extra reactions are called redundant and represent

the amount of degrees of indeterminacy).

SHEAR FORCE & BENDING MOMENT

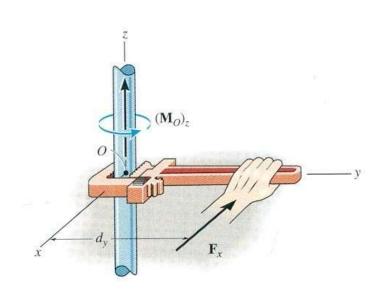
• In order to properly design a beam, it is important to know the *variation* of the shear and moment along its axis in order to find the points where these values are a maximum.

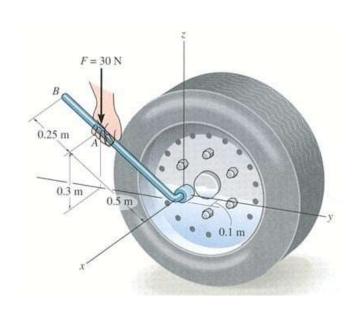


PRINCIPLE OF MOMENTS

- The moment of a force indicates the tendency of a body to turn about an axis passing through a specific point O.
- The principle of moments, which is sometimes referred to as *Varignon's Theorem* (Varignon, 1654 – 1722) states that *the moment of a* force about a point is equal to the sum of the moments of the force's components about the point.

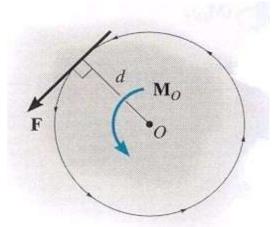
PRINCIPLE OF MOMENTS





In the 2-D case, the magnitude of the moment is:

 M_o = Force x distance

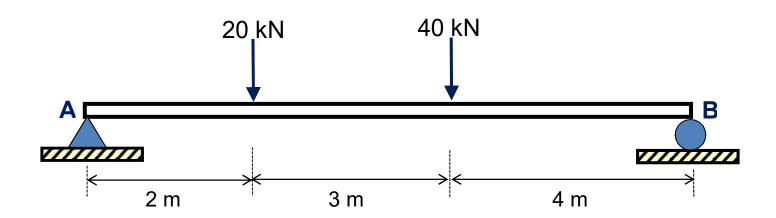


BEAM'S REACTION

- If a support prevents translation of a body in a particular direction, then the support exerts a force on the body in that direction.
- Determined using $\Sigma F_x = 0$, $\Sigma F_y = 0$ and $\Sigma M = 0$

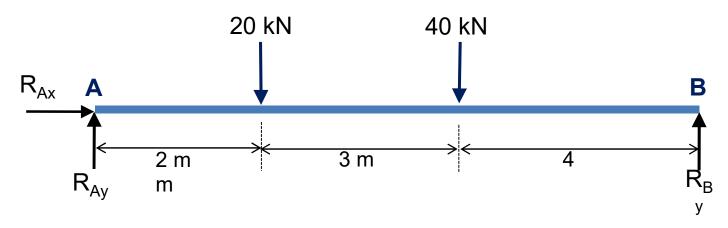
EXAMPLE 1

The beam shown below is supported by a pin at A and roller at B. Calculate the reactions at both supports due to the loading.



EXAMPLE 1 – Solution

Draw the free body diagram:



By taking the moment at

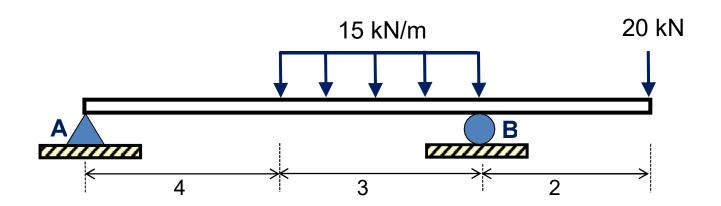
B,
$$\Sigma M_{\rm B} = 0$$

 $R_{\rm Ay} \times 9 - 20 \times 7 - 40 \times 4$
= 0
 $9R_{\rm Ay} = 140 + 160$
 $R_{\rm Ay} = 33.3 \text{ kN}$

$$\Sigma F_{y} = 0$$
 $\Sigma F_{x} = 0$ $R_{Ay} + R_{By} - 20 - 40$ $R_{Ax} = 0$ $R_{By} = 20 + 40 - 33.3$ $R_{By} = 26.7 \text{ kN}$

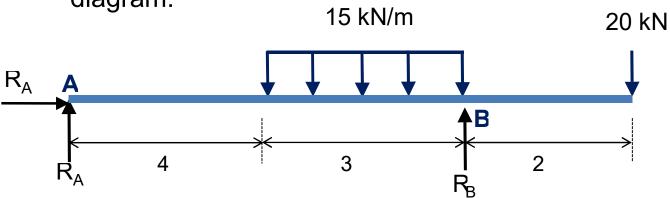
EXAMPLE 2

Determine the reactions at support A and B for the overhanging beam subjected to the loading as shown.



EXAMPLE 2 – Solution

Draw the free body diagram:



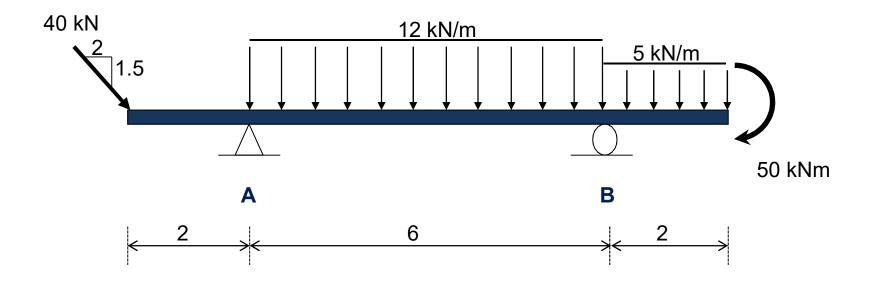
By taking the moment at A:

$$\Sigma M_A = 0$$

 $-R_{By} \times 7 + 20 \times 9 - (15 \times 3) \times 5.5$
 $= 0$
 $7R_{By} = 247.5 + 180$
 $R_{By} = 61.07 \text{ kN}$

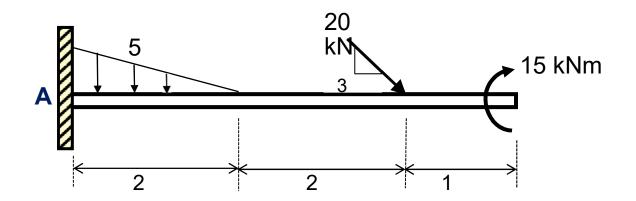
$$\Sigma F_{y} = 0$$
 $\Sigma F_{x} = 0$
 $R_{Ay} + R_{By} - 20 - 45$ $R_{Ax} = 0$
 $= 0$
 $R_{Ay} = 20 + 45 - 61.07$
 $R_{Ay} = 3.93 \text{ kN}$

CLASS EXERCISE – 5 mins?



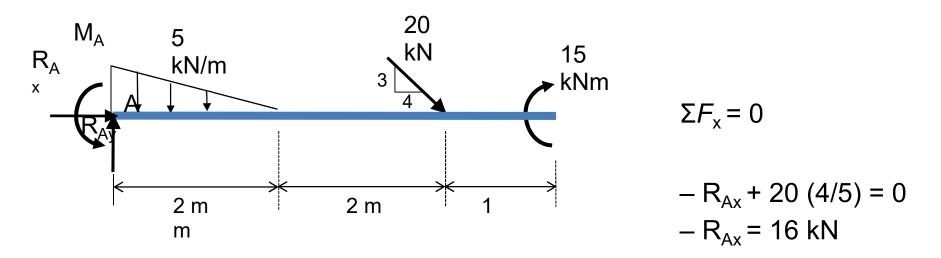
EXAMPLE 3

A cantilever beam is loaded as shown. Determine all reactions at support A.

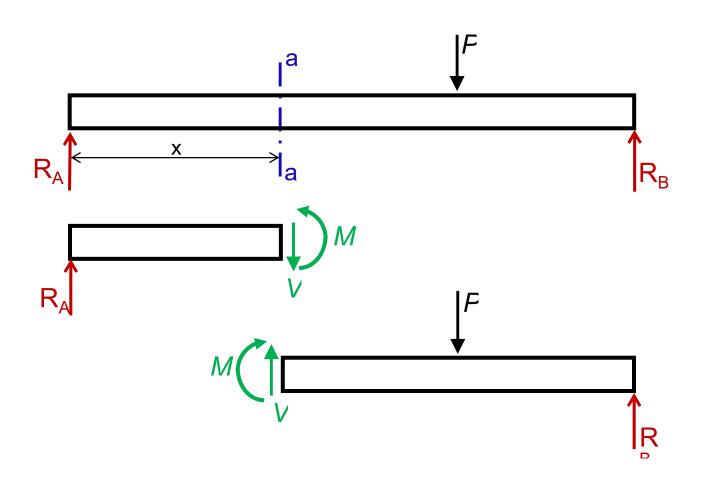


EXAMPLE 3 – Solution

Draw the free body diagram:



$$\Sigma F_{y} = 0$$
 $\Sigma M_{A} = 0$ $-M_{A} + 0.5(5)(2)(1/3)(2) + 20(3/5)(4) + 15 = 0$ $M_{A} = 3.3 + 48 + 15$ $R_{Ay} - 5 - 12 = 0$ $M_{A} = 66.3 \text{ kNm}$ $R_{Ay} = 17 \text{ kN}$



- **V** = shear force
 - = the force that tends to separate the member
 - = balances the reaction R_A
- **M** = bending moment
 - = the reaction moment at a particular point (section)
 - = balances the moment, $R_A \cdot x$

From the equilibrium equations of statics:

$$+ \int \Sigma F_{y} = 0; \qquad R_{A} - V = 0 \qquad \therefore V = R_{A}$$

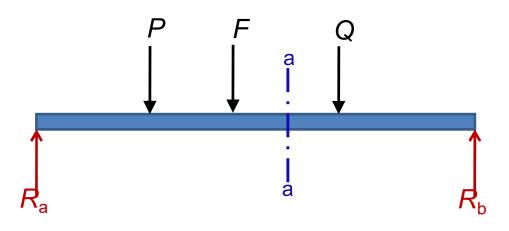
$$R_A - V = 0$$

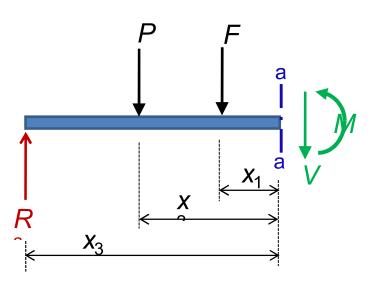
$$\therefore V = R_A$$

$$+\Sigma M_{\text{a-a}} = 0;$$
 $-M + R_{\text{A}} \cdot x = 0$ $\therefore M = R_{\text{A}} \cdot x$

$$-M + R_{\mathsf{A}} \cdot x = 0$$

$$\therefore M = R_A \cdot x$$





$$\Sigma F_{y} = 0$$

$$R_{a} - P - F - V = 0$$

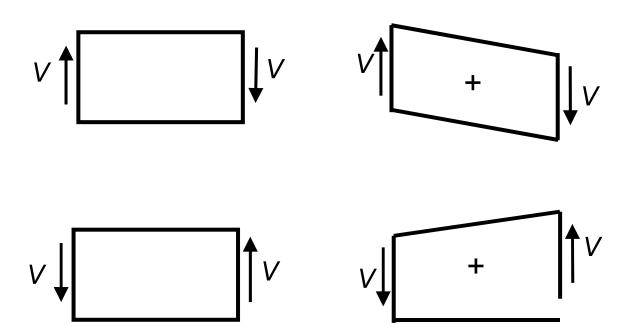
$$V = R_{a} - P - F$$

$$\Sigma M_{a-a} = 0$$

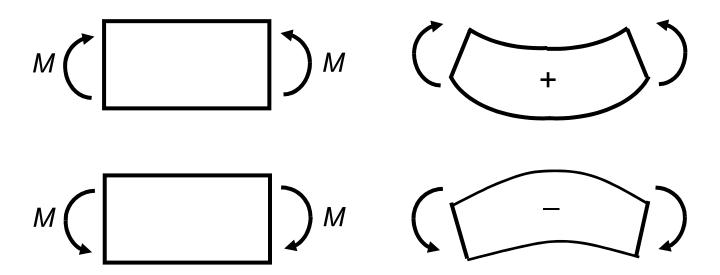
$$-M - F \cdot x_1 - P \cdot x_2 + R_a \cdot x_3 = 0$$

$$M = R_a \cdot x_3 - F \cdot x_1 - P \cdot x_2$$

Shape deformation due to shear force:



Shape deformation due to bending moment:

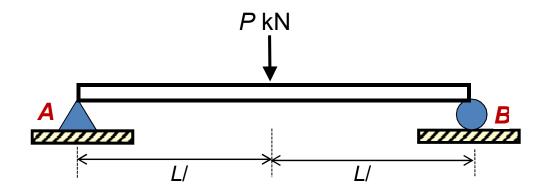


Sign Convention:

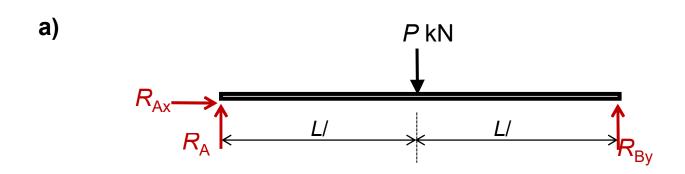
- Positive shear force diagram drawn ABOVE the beam
- Positive bending moment diagram drawn BELOW the beam

EXAMPLE 4

- a) Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure. Then, draw the shear force diagram (SFD) and bending moment diagram (BMD).
- b) If P = 20 kN and L = 6 m, draw the SFD and BMD for the beam.



EXAMPLE 4 – Solution



By taking the moment at A:

$$\Sigma M_{A} = 0$$

$$-R_{By} \times L + P \times L/2 = 0$$

$$R_{By} = P/2 \text{ kN}$$

$$\Sigma F_{y} = 0$$

$$R_{Ay} + R_{By} = 0$$

$$P R_{Ay} = P - 0$$

$$P/2$$

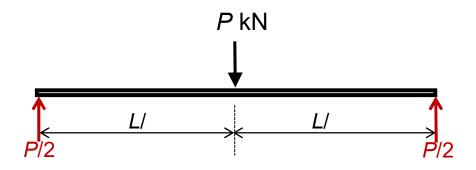
$$R_{Ay} = P/2$$

$$kN$$

$$\Sigma F_{x} = 0$$

$$R_{Ax} = 0$$

EXAMPLE 4 – Solution

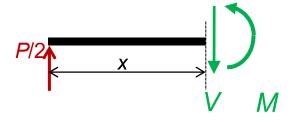


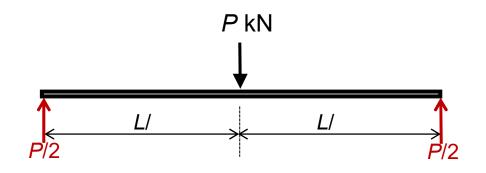
Between 0 < x < <u>L/2:</u>

$$\Sigma F_{y} = 0,$$
 $-V + P/2 = 0$ $V = P/2 \text{ kN}$

$$\Sigma M_{\text{a-a}} = 0,$$
 $-M + Px/2 = 0$ $M = Px/2 \text{ kNm}$

If x = 0 m, V = P/2 kN and M = 0 kNm If x = L/2 m, V = P/2 kN and M = PL/4 kNm





<u>Between *L*/2 < *x* < <u>*L*:</u></u>

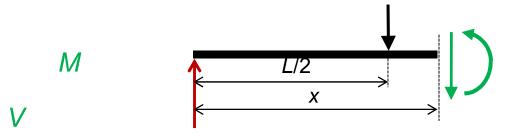
P kN

$$\Sigma F_{y} = 0,$$
 $-V + P/2 - P = 0$
 $V = -P/2 \text{ kN}$

If
$$x = L \text{ m}$$
, $V = -P/2 \text{ kN}$ and $M = 0 \text{ kNm}$

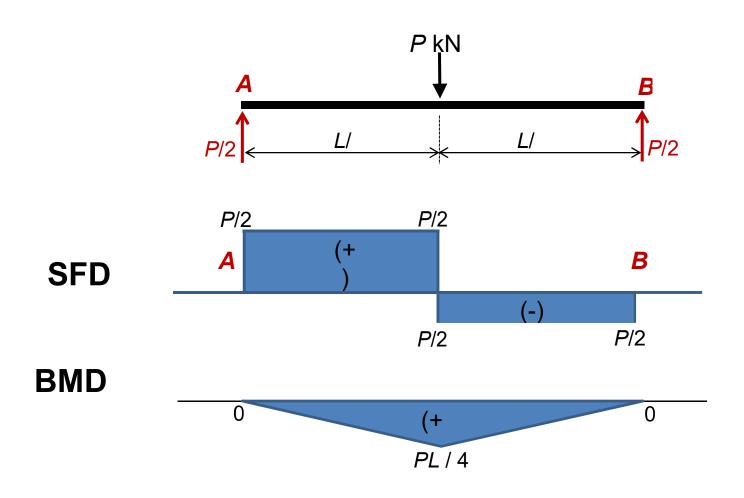
$$\Sigma M_{\text{a-a}} = 0,$$
 $-M + Px/2 - P(x - L/2)$
= 0 $M = PL/2 - Px/2 \text{ kNm}$

If
$$x = L/2$$
 m, $V = -P/2$ kN and $M = PL/4$ kNm

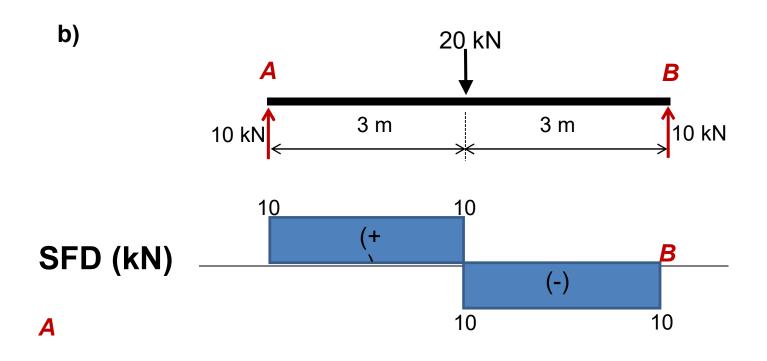


P/2

EXAMPLE 4 – Solution



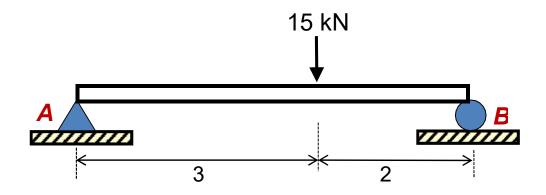
EXAMPLE 4 – Solution



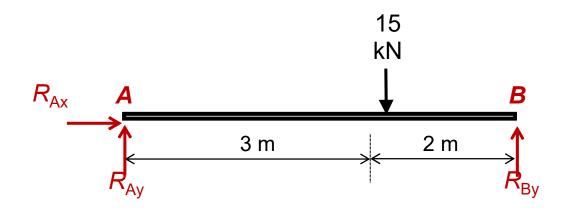


EXAMPLE 5

Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 5 – Solution



By taking the moment at

A:
$$\Sigma M_{A} = 0$$

$$-R_{\rm By}\times 5+15\times 3=0$$

$$R_{\rm By}$$
 = 9 kN

$$\Sigma F_{y} = 0$$

$$R_{Ay} + R_{By} = 15$$

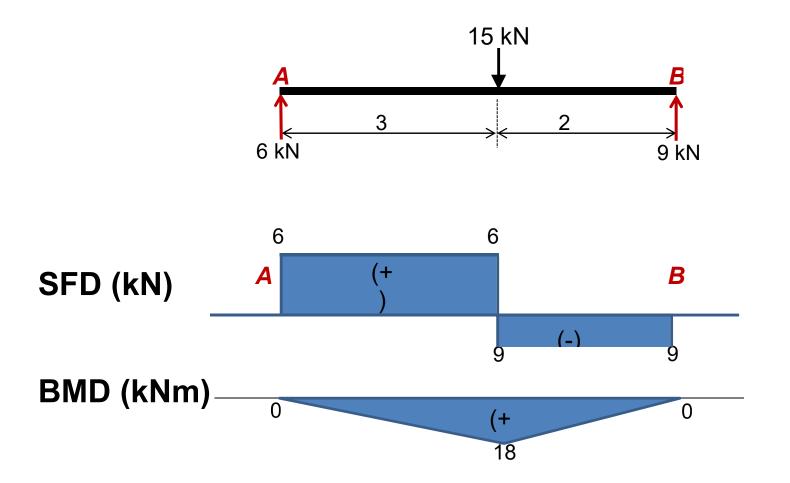
$$15 R_{Ay} = 15$$

$$-9 R_{Ay} = 6$$

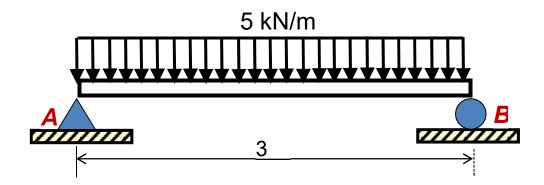
$$kN$$

$$\Sigma F_{x} = 0$$
$$R_{Ax} = 0$$

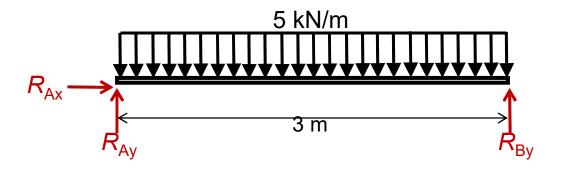
EXAMPLE 5 – Solution



Calculate the shear force and bending moment for the beam subjected to an uniformly distributed load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 6 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

 $-R_{By} \times 3 + 5 \times 3 \times 3/2 = 0$
 $R_{By} = 7.5 \text{ kN}$

$$\Sigma F_{y} = 0$$

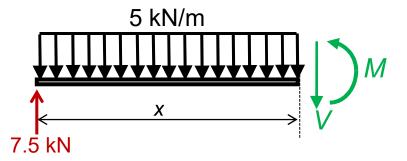
 $R_{Ay} + R_{By} = 5 \times 3$
 $3 R_{Ay} = 15 - 7.5$
 kN

$$\Sigma F_{x} = 0$$

$$R_{Ax} = 0$$

EXAMPLE 6 – Solution

These results for V and M can be checked by noting that dV/dx = -w. This is correct, since positive w acts downward. Also, notice that dM/dx = V. The maximum moments occurs when dM/dx = V = 0.

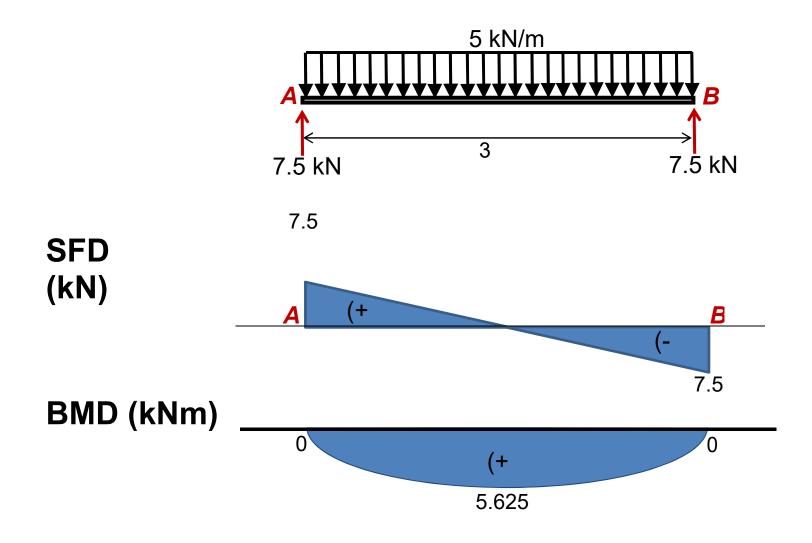


$$\Sigma M_{a-} = 0,$$
 $-M + 7.5x - 5x (x/2)$ $M = \text{maximum}$ $\frac{dM}{dx} = 0$ when $\frac{dM}{dx} = 0$

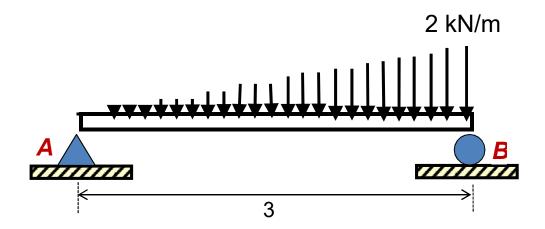
$$M = 7.5x - \frac{dM}{5x^2/2} = 7.5 - 5x = 0$$
 $\therefore x = 1.5 \text{ m}$

Therefore, $M_{\text{max}} = 5.625 \text{ kNm}$

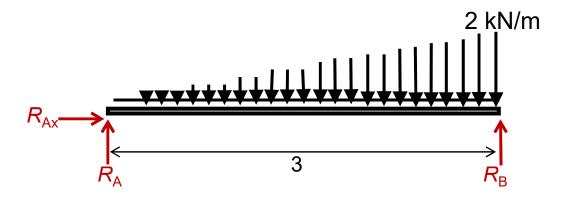
EXAMPLE 6 – Solution



Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 7 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$2 \times 3/2 \times 3 \times 2/3 - R_{By} \times 3$$

$$= 0$$

$$R_{By} = 2 \text{ kN}$$

$$\Sigma F_{y} = 0$$

$$R_{Ay} + R_{By} = 2 \times 3/2$$

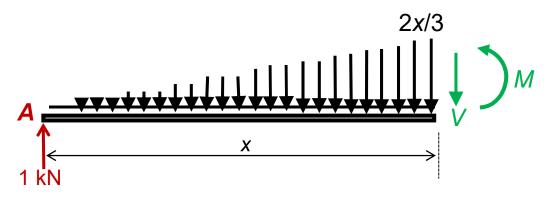
$$R_{Ay} = 3 - 2$$

$$R_{Ay} = 1 \text{ kN}$$

 $\Sigma F_x = 0$

 $R_{Ax} = 0$

EXAMPLE 7 – Solution



$$1 - 2x/3(x)(1/2) - V = 0$$

$$V = 1 - 2x^{2}/6$$
If $x = 0$, $V = 1$ kN and $x = 3$, $V = -2$ kN
$$-M + 1 \times x - 2x/3(x)(1/2)(x/3) = 0$$

M = maximum when

$$\frac{dM}{dx} = 0$$

$$\frac{dM}{dx} = 1 - \frac{3x^2}{9} = 0$$
$$x^2 = \frac{9}{3}$$

$$\sqrt{3}$$

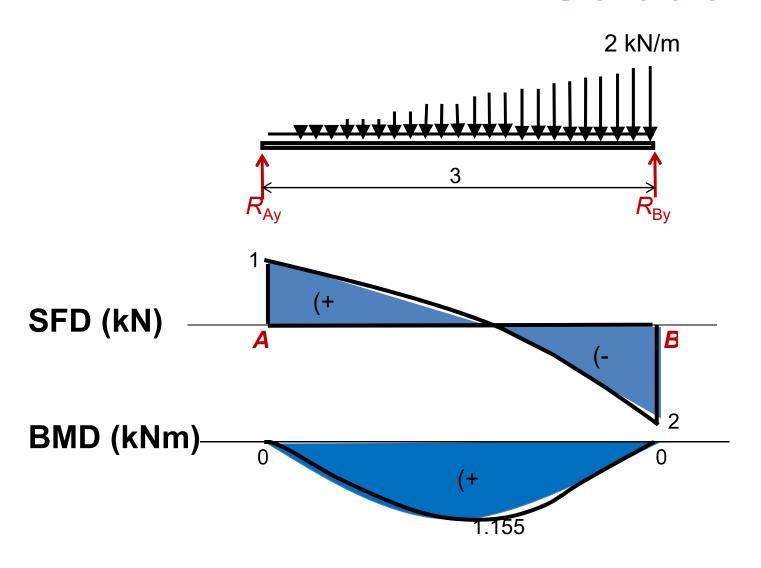
$$M = x -$$

$$x^{3}/9$$

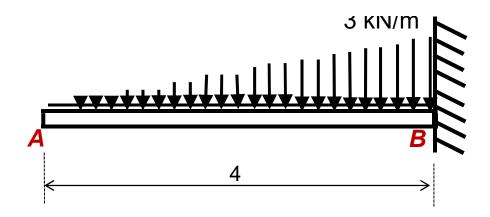
x =

Theref ore, *M*_{max} = 1.155 kNm

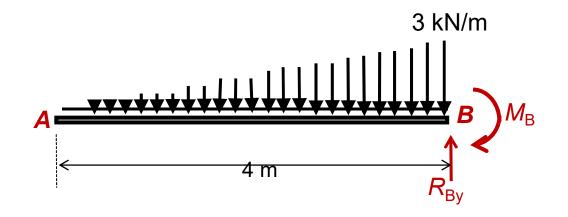
EXAMPLE 7 – Solution



Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 8 – Solution



By taking the moment at

B:
$$\Sigma M_{\rm B} = 0$$

$$M_{\rm B} = 3 \times 4/2 \times 4/3$$

$$M_{\rm B}$$
 = 8 kNm

$$\Sigma F_y = 0$$

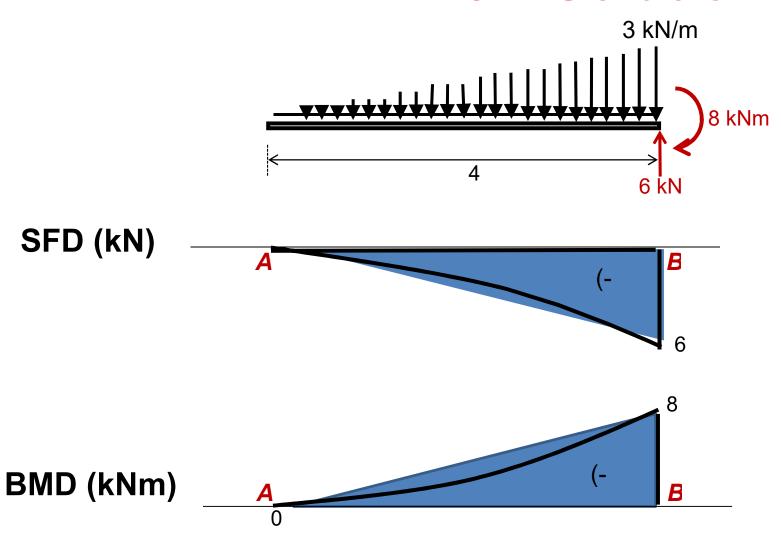
$$R_{\rm By}$$
 = 3 \times

$$R_{\rm By}$$
 = 6 kN

$$\Sigma F_x = 0$$

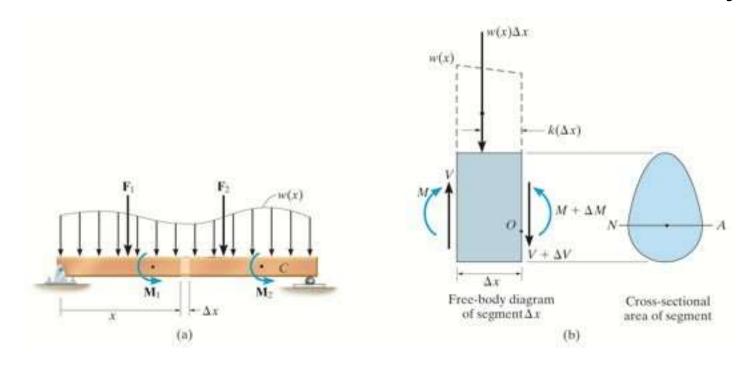
$$R_{\rm Bx} = 0$$

EXAMPLE 8 – Solution



RELATIONSHIP BETWEEN LOAD, SHEAR FORCE & BENDING MOMENT

When a beam is subjected to two or more concentrated or distributed load, the way to



calculate and draw the SFD and BMD may not be the same as in the previous situation.

REGION OF DISTRIBUTED LOAD

$$\Sigma F_{y} = 0; V - w(x)\Delta x - (V + \Delta V) = 0$$
$$\Delta V = w(x)\Delta x$$

$$\Sigma M_0 = 0;$$

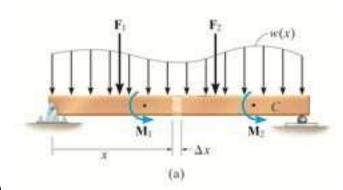
$$-V \Delta x - M + w(x) \Delta x [k \Delta x] + (M + \Delta M) = 0$$

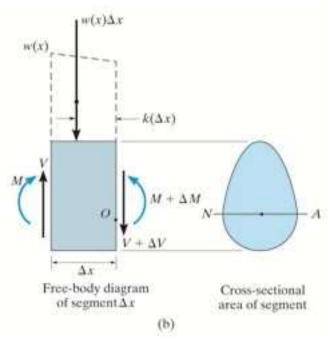
$$\Delta M = V \Delta x - w(x) k \Delta x^2$$

Dividing by Δx and taking the limit as Δx = the above two equations become:

Slope of moment diagram at each the shear
$$\frac{dV}{dx} = -w(x)$$
 diagram at $\frac{dV}{dx}$ each point

Slope of





nsity at each point

$$\frac{dM}{dx} = V$$
 Shear at each point

n

е

REGION OF DISTRIBUTED LOAD

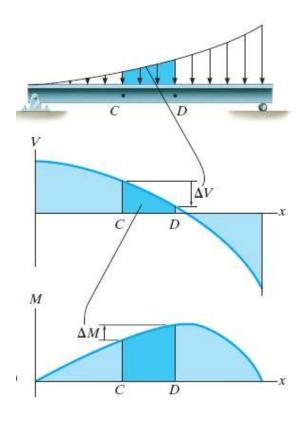
 We can integrate these areas between any two points to get change in shear and moment.

Change in shear
$$\Delta V = -\int w(x)dx$$

Area under distributed loading

Change in moment
$$\Delta M = \int V(x) dx$$

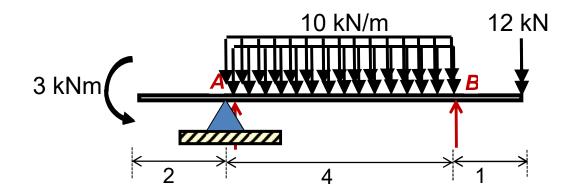
Area under shear diagram



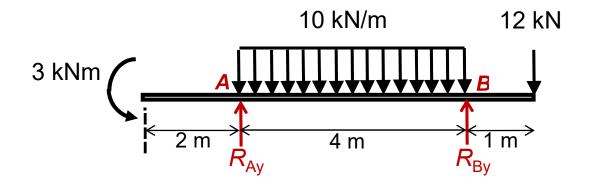
USEFUL TIPS...

- Slope of bending moment always determined by the shape of shear force lines. The changes in slope (sagging or hogging also depends on the changes in shear force values)
- When shear force intersects BMD axis, there is a maximum moment
- When SF maximum, BM minimum and vice versa
- SFD and BMD always start and end with zero values (unless at the point where there is a moment/couple)
- When a moment/couple acting:
 - Clockwise (\downarrow) (+), Anticlockwise (\uparrow) (-)

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 9 – Solution

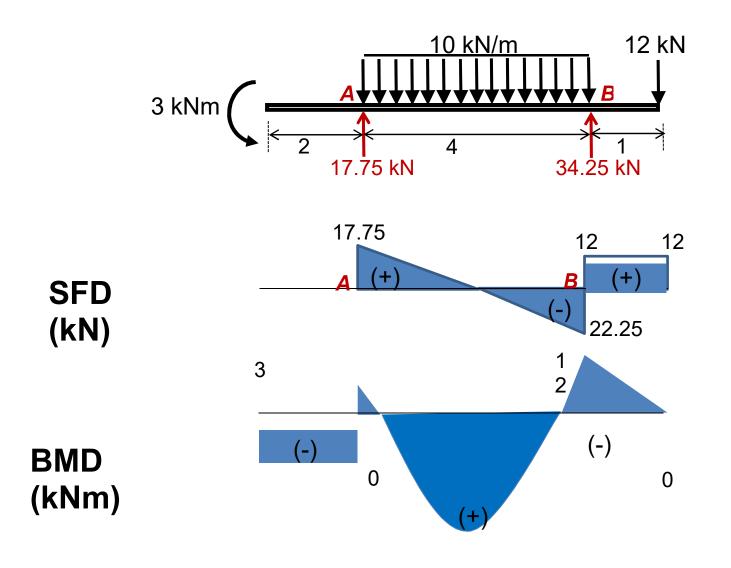


By taking the moment at A:

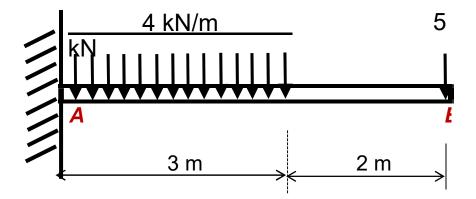
$$\Sigma M_A = 0$$
 $-R_{By} \times 4 - 3 + 10 \times 4 \times 4/2 + 12 \times 5$
 $= 0$
 $R_{By} = 34.25 \text{ kN}$

$$\Sigma F_{y} = 0$$
 $\Sigma F_{x} = 0$ $R_{Ay} + R_{By} = 10 \times 4 + R_{Ax} = 0$ $R_{Ay} = 17.75 \text{ kN}$

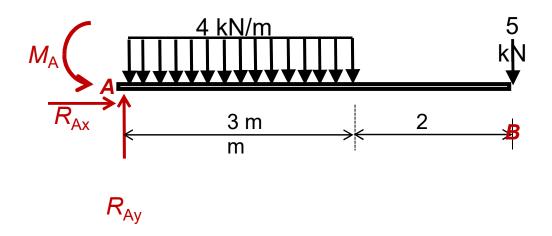
EXAMPLE 9 – Solution



Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 10 – Solution



By taking the moment at

A:
$$\Sigma M_A = 0$$

 $-M_A + 4 \times 3 \times 3/2 + 5 \times 5$
 $= 0$
 $M_A = 43 \text{ kNm}$

$$\Sigma F_{y} = 0$$

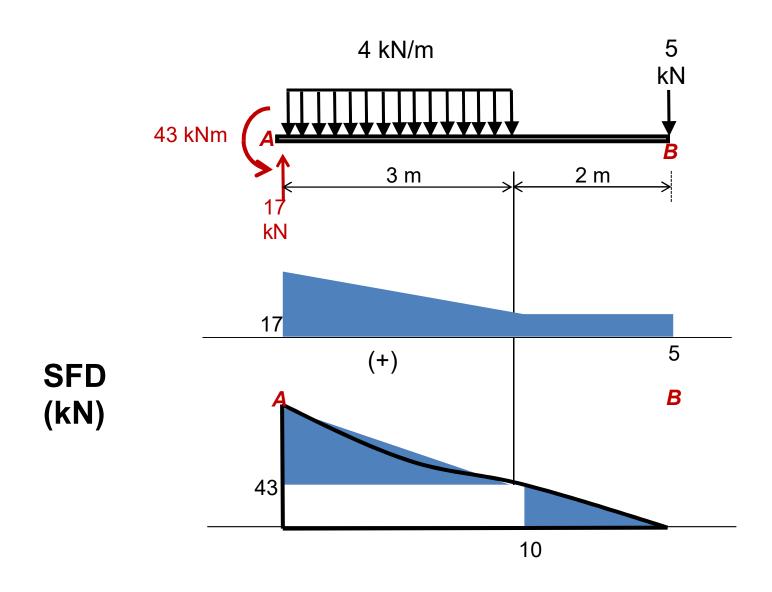
$$R_{Ay} = 4 \times 3 + 5$$

$$R_{Ay} = 17 \text{ kN}$$

 $\Sigma F_x = 0$

 $R_{Ax} = 0$

EXAMPLE 10 – Solution

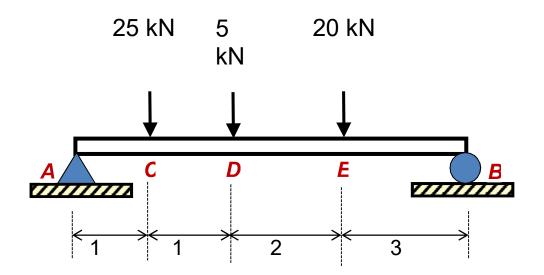


BMD (kNm)

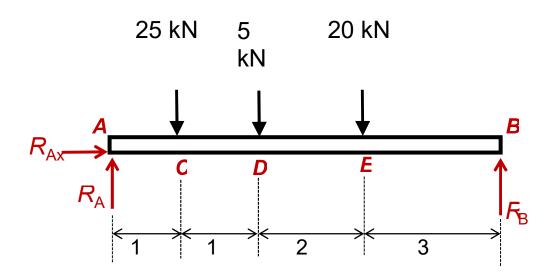
(-)

0

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 11 – Solution



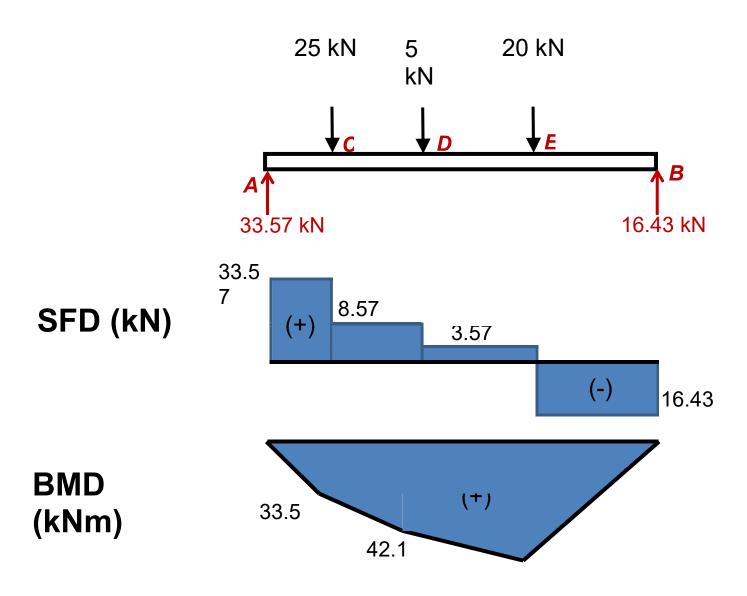
By taking the moment at A:

$$\Sigma M_A = 0$$

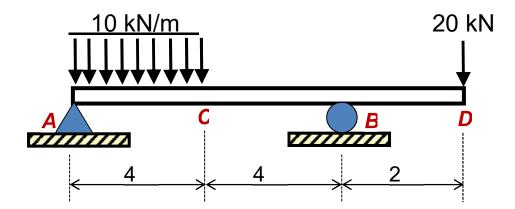
 $25 \times 1 + 5 \times 2 + 20 \times 4 - R_{By} \times 7$
= 0
 $R_{By} = 16.43 \text{ kN}$

$$\Sigma F_{y} = 0$$
 $\Sigma F_{x} = 0$ $R_{Ay} + R_{By} = 25 + 5 + R_{Ax} = 0$ $R_{Ay} = 33.57 \text{ kN}$

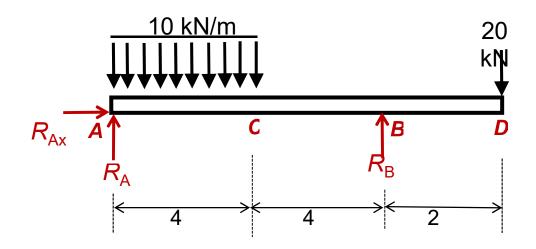
EXAMPLE 11 – Solution



Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 12 – Solution

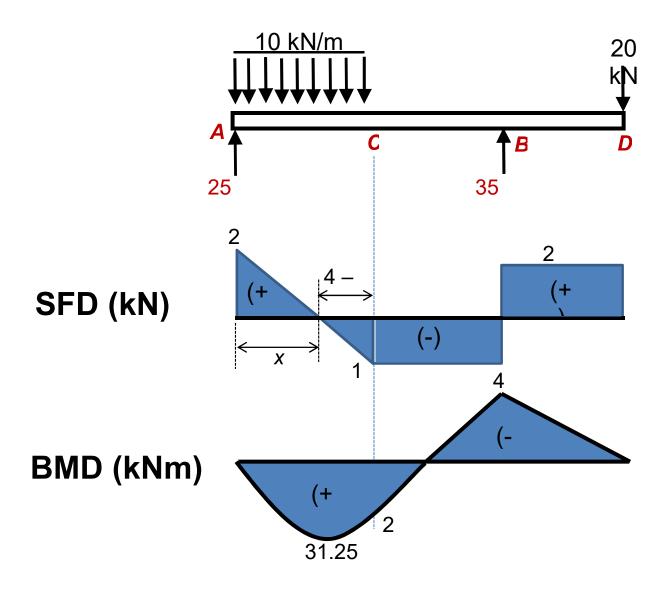


By taking the moment at A:

$$\Sigma M_{A} = 0$$
 $10 \times 4 \times 2 + 20 \times 10 - R_{By} \times 8$
 $= 0$
 $R_{By} = 35 \text{ kN}$

$$\Sigma F_{y} = 0$$
 $\Sigma F_{x} = 0$ $R_{Ay} + R_{By} = 10 \times 4 + R_{Ax} = 0$ $R_{Ay} = 60 - 35$ $R_{Ay} = 25 \text{ kN}$

EXAMPLE 12 – Solution



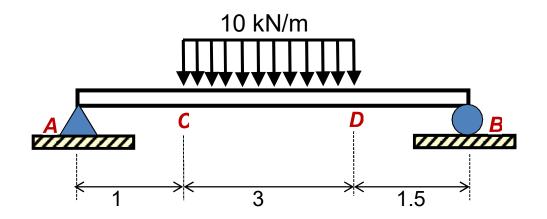
$$\frac{x}{25} = \frac{4 - x}{15}$$

$$15x = 100 - 25x$$

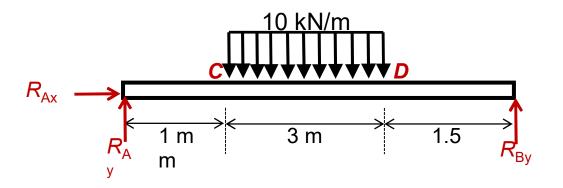
$$40x = 100$$

$$x = 2.5$$

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



EXAMPLE 13 – Solution



By taking the moment at

A:
$$\Sigma M_A = 0$$

$$10 \times 3 \times 2.5 - R_{By} \times 5.5 =$$

U

$$R_{\rm By} = 13.64 \; \rm kN$$

$$\Sigma F_{v} = 0$$

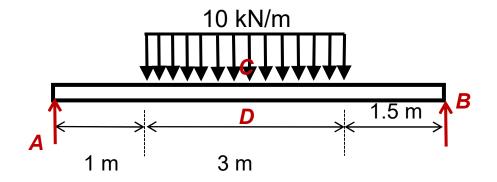
$$R_{Ay} + R_{By} = 10$$

$$\times$$
 3 R_{Ay} = 30 $-$

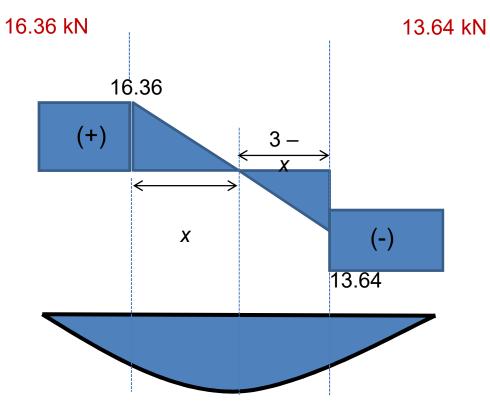
13.64
$$R_{AV}$$
 =

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$







$$\frac{x}{16.36} = \frac{3 - x}{13.64}$$

$$13.64x = 49.08 - 16.36x$$

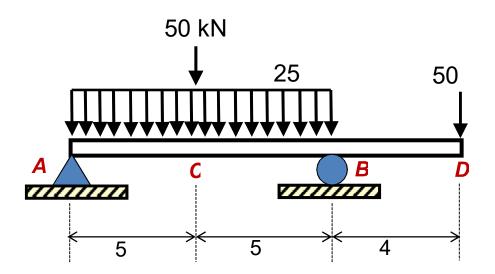
$$30x = 49.08$$

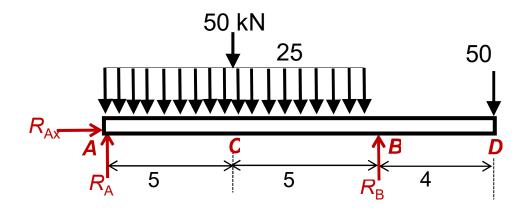
$$x = 1.636$$

BMD (kNm) 16.3 6 (+)

EXAMPLE 14

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).





By taking the moment at A:

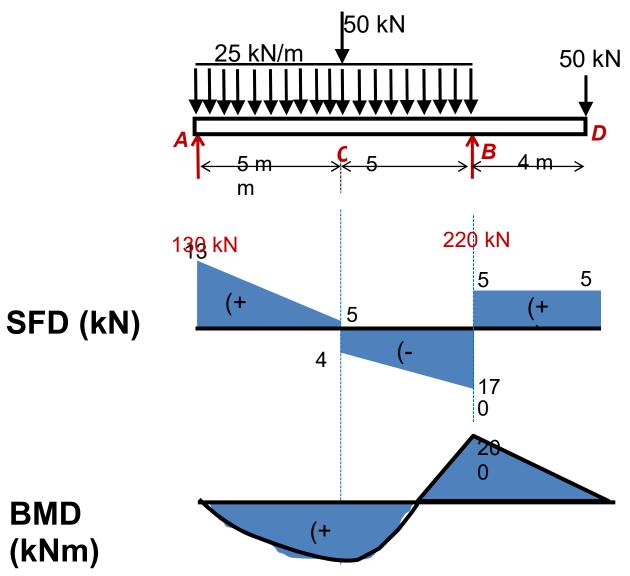
$$\Sigma M_A = 0$$

 $25 \times 10 \times 5 + 50 \times 5 + 50 \times 14 - R_{By} \times 10 = 0$
 $R_{By} = 220 \text{ kN}$

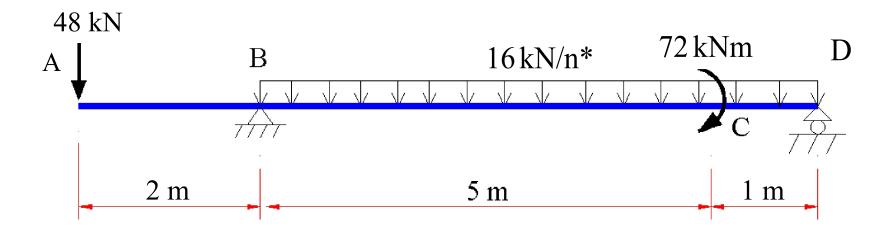
$$\Sigma F_y = 0$$

 $R_{Ay} + R_{By} = 25 \times 10 + 50 + 50$
 $R_{Ay} = 130 \text{ kN}$

$$\Sigma F_{x} = 0$$
$$R_{Ax} = 0$$



CLASS EXERCISE



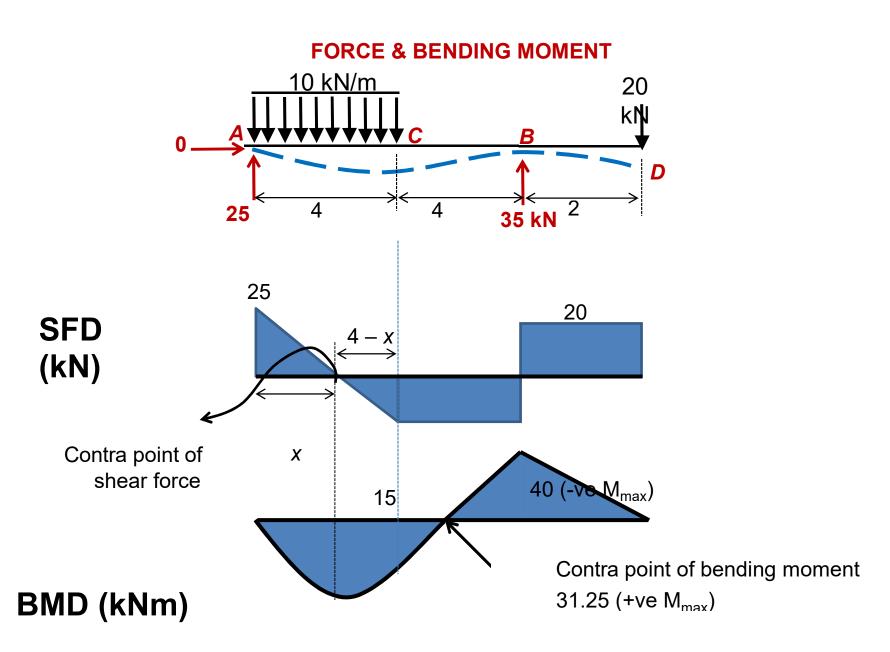
CONTRA Flexture POINT OF SHEAR FORCE & BENDING MOMENT

- Contra flexure point is a place where positive shear force/bending moment shifting to the negative region or viceversa.
- Contra flexure point for shear: V = 0
- Contra flexure point for moment: M = 0
- When shear force is zero, the moment is

maximum.

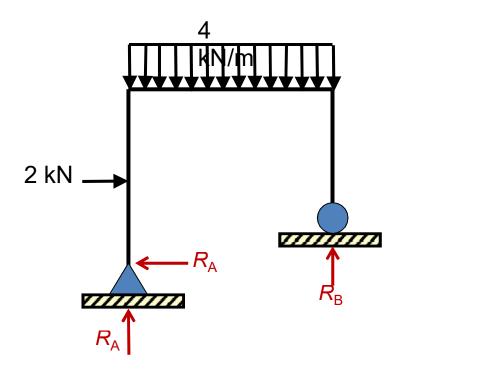
 Maximum shear force usually occur at the support / concentrated load.

CONTRA FLEXTURE POINT OF SHEAR



STATICALLY DETERMINATE FRAMES

 For a frame to be statically determinate, the number of unknown (reactions) must be able to solved using the equations of equilibrium.



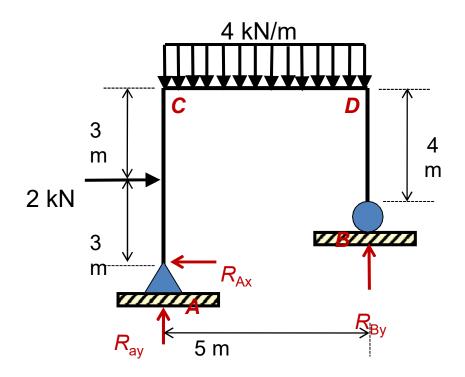
$$\Sigma M_{A} = 0$$

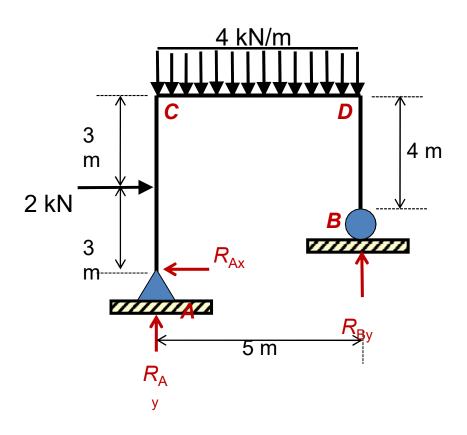
$$\Sigma F_{y} = 0$$

$$\Sigma F_{x} = 0$$

EXAMPLE 15

Calculate the shear force and bending moment for the frame subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).





$$\Sigma M_A = 0$$

 $4 \times 5 \times 2.5 + 2 \times 3 - R_{By} \times 5 = 0$
 $R_{By} = 11.2 \text{ kN}$

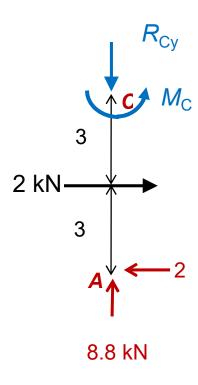
$$\Sigma F_{y} = 0$$

$$R_{Ay} + R_{By} = 4 \times 5$$

$$R_{Ay} = 8.8 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 2 \text{ kN}$$

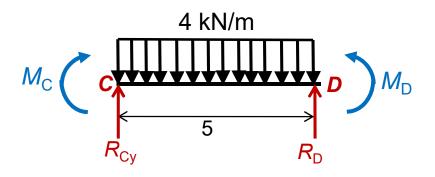


$$\Sigma M_{\rm A} = 0: 2 \times 3 - M_{\rm C} = 0$$

$$M_C = 6 \text{ kNm}$$

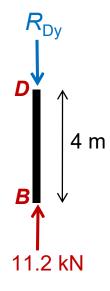
$$\Sigma F_y = 0$$

$$\therefore R_{cy} = 8.8 \text{ kN}$$

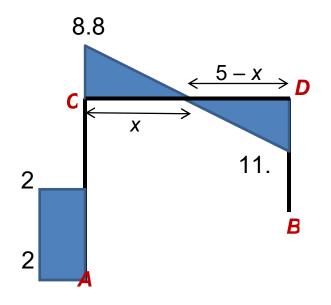


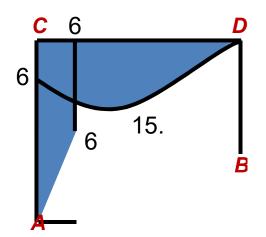
$$\Sigma F_y = 0$$
: $R_{Cy} + R_{Dy} = 4 \times 5$
 $R_{Dy} = 20 - 8.8 = 11.2 \text{ kN}$

$$\Sigma M_{\rm C} = 0$$
:
 $M_{\rm C} + 4 \times 5 \times 2.5 - R_{\rm Dy} \times 5 - M_{\rm D} = 0$
 $M_{\rm D} = 0 \text{ kNm}$



$$\Sigma F_{y} = 0$$
:
 $R_{Dy} = 11.2 \text{ kN}$





$$\frac{x}{8.8} = \frac{5 - x}{11.2}$$

$$11.2x = 44 - 8.8x$$

$$20x = 44$$

$$x = 2.2$$

$$M_{\text{max}} = 8.8 \times 2.2 \times 0.5 + 6 = 15.7 \text{ kNm}$$

SFD (kN)

BMD (kNm)